

1614A.1.30 ASCE 7, Section 18.2.4. *Modify ASCE 7, Section 18.2.4, second sentence as follows:*

Regardless of the analysis method used, the peak dynamic response of the structure and elements of the damping system shall be confirmed by using the nonlinear response history procedure if the structure is located at a site with S_1 greater than or equal to 0.6.

1614A.1.31 ASCE 7, Section 18.9.2. *Modify ASCE 7, Section 18.9.2 by adding the following:*

Required Tests of Energy Dissipation Devices - Production Tests. *Production testing and associated acceptance criteria shall be as approved by the enforcement agent.*

Notation [For DSA-SS]:

Authority: Education Code Sections 17310, 81142; Health & Safety Code Section 16022

Reference(s): Education Code Sections 17280 - 17317, and 811130 - 81149; Health & Safety Code Sections 16000 - 16023

Notation [For OSHPD]:

Authority: Health and Safety Code Section 129850

Reference: Health and Safety Code Sections 1275, 129850 and 129790

CHAPTER 17A - STRUCTURAL TESTS AND SPECIAL INSPECTIONS

2001 CBC	PROPOSED ADOPTION	OSHPD		DSA-SS	Comments
		1	4		
	Adopt entire chapter without amendments				
	Adopt entire chapter with amendments listed below	X	X	X	
	Adopt only those sections listed below				
	1701A.1.1 CA	X	X	X	
	1701A.1.2 CA	X	X	X	
1701A.1.2	1701A.4 CA	X	X		Relocated existing California Building Standards into IBC format
1701A.1.1	1701A.5 CA			X	Relocated existing California Building Standards into IBC format
	1702A.1	X	X	X	Editorial
	1704A.1	X	X	X	Editorial
	1704A.1.1	X	X	X	
1701A.3.2	1704A.1.2	X	X	X	Relocated existing California Building Standards into IBC format
	1704A.2.1	X	X	X	
	Table 1704A.3				Editorial - Heading
2231A.5 CA	1704A.3.1.1 CA	X	X	X	Relocated existing California Building Standards into IBC format

2231A.4 CA	1704A.3.2.1 CA	X	X	X	Relocated existing California Building Standards into IBC format
2231A.5 CA	1704A.3.2.2 CA	X	X	X	Relocated existing California Building Standards into IBC format
2231A.5 CA	1704A.3.2.3 CA	X	X	X	Relocate existing California Building Standards into IBC format
	1704A.4	X	X	X	
1701A.5, Item #18 CA	Table 1704A.4, Item #12	X	X	X	Relocated existing California Building Standards into IBC format
1929A.12 CA	1704A.4.2 CA	X	X	X	Relocated existing California Building Standards into IBC format
1929A.4 CA	1704A.4.3 CA	X	X	X	Relocated existing California Building Standards into IBC format
1929A.5 CA	1704A.4.4 CA	X	X	X	Relocated existing California Building Standards into IBC format
1929A.9 CA	1704A.4.5 CA	X	X	X	Relocated existing California Building Standards into IBC format
1905A.7.1 CA	1704A.4.6 CA	X	X	X	Relocated existing California Building Standards into IBC format
1929A.7 CA	1704A.4.7 CA	X	X	X	Relocated existing California Building Standards into IBC format
	1704A.5	X	X	X	
	1704A.5.1	X	X	X	
1701A.5.18 CA	Table 1704A.5.1, Item #7	X	X	X	Relocated existing California Building Standards into IBC format
	1704A.5.2	X	X	X	
	1704A.5.3	X	X	X	
1701A.5.18 CA	Table 1704A.5.3, Item #5	X	X	X	Relocated existing California Building Standards into IBC format
	1704A.6	X	X	X	
	1704A.6.2	X	X	X	
2337A.1 CA	1704A.6.2.1 CA	X	X	X	Relocated existing California Building Standards into IBC format
2337A.3 CA	1704A.6.2.2 CA	X	X	X	Relocated existing California Building Standards into IBC format
2337A.2 CA	1704A.6.3 CA	X	X	X	Relocated existing California Building Standards into IBC format
3301.1 CA	1704A.7.1 CA	X	X	X	Relocated existing California Building Standards into IBC format
1809A.6 CA	1704A.8.1 CA	X	X	X	Relocated existing California Building Standards into IBC format
1809A.7.1 CA	1704A.9.1	X	X	X	Relocated existing California Building Standards into IBC format

1929A.10 CA	1704A.15 CA	X	X	X	Relocated existing California Building Standards into IBC format
1929A.11.2 CA	1704.15.1 CA	X	X	X	Relocated existing California Building Standards into IBC format
1701A.5.8 CA	1704A.16 CA	X	X	X	Relocated existing California Building Standards into IBC format
	1707A.3	X	X	X	
	1707A.7	X	X	X	
	1707A.10	X	X	X	
	1708A.1.1	X	X	X	
	Table 1708A.1.2	X	X	X	
	1708A.1.2	X	X	X	
	1708A.1.3	X	X	X	
	Table 1708A.1.4	X	X	X	
	1708A.1.4	X	X	X	
1702A.2 CA	1709A.2 CA	X	X	X	Relocated existing California Building Standards into IBC format
1702A.2 CA	1709A.3 CA	X	X	X	Relocated existing California Building Standards into IBC format
	1711A.1 CA	X	X	X	Editorial

REPEAL OF EXISTING CALIFORNIA AMENDMENTS IN PART OR IN WHOLE THAT ARE NO LONGER NECESSARY AS FOLLOWS:

~~2001 CBC SECTION 1701A.2 — Project and Special Inspector:~~ Repeal all amendments in this section and all subsections.

~~2001 CBC SECTION 1701A.3 — Duties and responsibilities of Project and Special Inspector:~~ Repeal all amendments in this section and all subsections.

~~2001 CBC SECTION 1701A.4 — Standards of Quality:~~ Repeal all amendments in this section.

~~2001 CBC SECTION 1701A.5 — TYPES OF WORK REQUIRING CONSTANT PRESENCE OF THE SPECIAL INSPECTOR:~~ Repeal all amendments in this section.

~~2001 CBC SECTION 1703A — NONDESTRUCTIVE TESTING:~~ Repeal all amendments in this section and all subsections.

~~2001 CBC SECTION 1704A — PREFABRICATED CONSTRUCTION:~~ Repeal all amendments in this section and all subsections.

EXPRESS TERMS

SECTION 1701A - GENERAL

1701A.1 Scope. The provisions of this chapter shall govern the quality, workmanship and requirements for materials covered. Materials of construction and tests shall conform to the applicable standards listed in this code.

1701A.1.1 Application *The scope of application of Chapter 17A is as follows:*

- 1. Structures regulated by the Division of the State Architect-Structural Safety (DSA-SS), which include those applications listed in Section 109.2 These applications include public elementary and secondary schools, community colleges and state-owned or state-leased essential services buildings*
- 2. Structures regulated by the Office of Statewide Health Planning and Development (OSHDP), which include those applications listed in Section 110.1, and 110.4. These applications include hospitals, skilled nursing facilities, intermediate care facilities and correctional treatment centers.*

Exception: [For OSHDP 2] *Single-story Type V skilled nursing or intermediate care facilities utilizing wood-frame or light-steel-frame construction as defined in Health and Safety Code Section 129725, which shall comply with CBC Chapter 17 and any applicable amendments therein.*

1701A.1.2 Amendments in this chapter. *DSA - SS and OSHDP adopt this chapter and all amendments.*

Exception: *Amendments adopted by only one agency appear in this chapter preceded with the appropriate acronym of the adopting agency, as follows:*

- 1. Division of the State Architect - Structural Safety:
[DSA-SS] - For applications listed in Section 109.2*
- 2. Office of Statewide Health Planning and Development:
[OSHDP 1] - For applications listed in Section 110.1
[OSHDP 4] - For applications listed in Section 110.4*

1701A.2 New materials. New building materials, equipment, appliances, systems or methods of construction not provided for in this code, and any material of questioned suitability proposed for use in the construction of a building or structure, shall be subjected to the tests prescribed in this chapter and in the approved rules to determine character, quality and limitations of use.

1701A.3 Used materials. The use of second-hand materials that meet the minimum requirements of this code for new materials shall be permitted.

1701A.4 (Relocated from 1701A.1.2, CBC 2001) **[For OSHDP 1 and 4]** *In addition to the project inspector inspector(s) of record required by Title 24, Part 1, Section 7-144, the hospital the owner or registered design professional in general responsible charge acting as the owner's agent shall employ one or more special inspectors who shall provide inspections during construction on the types of work listed under Section 1701A.5, Chapters 17A, 18A, 19A, 20A 20, 21A, 22A, 23A 23, 25, 34A, and noted in the special test, inspection and observation plan required by Sections 7-141, 7-145 and 7-149 of Title 24, Part 1, of the California Building Standards Administrative Code.*

1701A.5 (Relocated from 1701A.1.1, CBC 2001) **[DSA-SS]** *In addition to the project inspector required by Title 24, Part 1, Section 4-333, the school district owner shall employ one or more special inspectors who shall provide inspections during construction on the types of work listed under Chapters 17A, 18A, 19A, 20, 21A, 22A, 23, 25, 34, and noted in the special test, inspection and observation plan required by Sections 4-335 of Title 24, Part 1, of the California Building Standards Administrative Code.*

SECTION 1702A - DEFINITIONS

1702A.1 General. The following words and terms shall, for the purposes of this chapter and as used elsewhere in this code, have the meanings shown herein.

APPROVED AGENCY. An established and recognized agency regularly engaged in conducting tests or furnishing

inspection services, when such agency has been approved.

APPROVED FABRICATOR. An established and qualified person, firm or corporation approved by the building official pursuant to Chapter 17A of this code.

CERTIFICATE OF COMPLIANCE. A certificate stating that materials and products meet specified standards or that work was done in compliance with approved construction documents.

DESIGNATED SEISMIC SYSTEM. Those architectural, electrical and mechanical systems and their components that require design in accordance with Chapter 13 of ASCE 7 and for which the component importance factor, I_p , is greater than 1 in accordance with Section 13.1.3 of ASCE 7.

FABRICATED ITEM. Structural, load-bearing or lateral load-resisting assemblies consisting of materials assembled prior to installation in a building or structure or subjected to operations such as heat treatment, thermal cutting, cold working or reforming after manufacture and prior to installation in a building or structure. Materials produced in accordance with standard specifications referenced by this code, such as rolled structural steel shapes, steel-reinforcing bars, masonry units and wood structural panels shall not be considered "fabricated items."

INSPECTION CERTIFICATE. An identification applied on a product by an approved agency containing the name of the manufacturer, the function and performance characteristics, and the name and identification of an approved agency that indicates that the product or material has been inspected and evaluated by an approved agency (see Section 1703A.5 and "Label," "Manufacturer's designation" and "Mark").

LABEL. An identification applied on a product by the manufacturer that contains the name of the manufacturer, the function and performance characteristics of the product or material, and the name and identification of an approved agency and that indicates that the representative sample of the product or material has been tested and evaluated by an approved agency (see Section 1703A.5 and "Inspection certificate," "Manufacturer's designation" and "Mark").

MAIN WIND-FORCE-RESISTING SYSTEM. An assemblage of structural elements assigned to provide support and stability for the overall structure. The system generally receives wind loading from more than one surface.

MANUFACTURER'S DESIGNATION. An identification applied on a product by the manufacturer indicating that a product or material complies with a specified standard or set of rules (see also "Inspection certificate," "Label" and "Mark").

MARK. An identification applied on a product by the manufacturer indicating the name of the manufacturer and the function of a product or material (see also "Inspection certificate," "Label" and "Manufacturer's designation").

SPECIAL INSPECTION. Inspection as herein required of the materials, installation, fabrication, erection or placement of components and connections requiring special expertise to ensure compliance with approved construction documents and referenced standards (see Section 1704A).

SPECIAL INSPECTION, CONTINUOUS. The full-time observation of work requiring special inspection by an approved special inspector who is present in the area where the work is being performed.

SPECIAL INSPECTION, PERIODIC. The part-time or intermittent observation of work requiring special inspection by an approved special inspector who is present in the area where the work has been or is being performed and at the completion of the work.

SPRAYED FIRE-RESISTANT MATERIALS. Cementitious or fibrous materials that are spray applied to provide fire-resistant protection of the substrates.

STRUCTURAL OBSERVATION. The visual observation of the structural system by a registered design professional for general conformance to the approved construction documents at significant construction stages and at completion of the structural system. Structural observation does not include or waive the responsibility for the inspection required by Section 109, *Appendix Chapter 1*, 1704A or other sections of this code.

SECTION 1703A APPROVALS

1703A.1 Approved agency. An approved agency shall provide all information as necessary for the building official to determine that the agency meets the applicable requirements.

1703A.1.1 Independent. An approved agency shall be objective and competent. The agency shall also disclose possible conflicts of interest so that objectivity can be confirmed.

1703A.1.2 Equipment. An approved agency shall have adequate equipment to perform required tests. The equipment shall be periodically calibrated.

1703A.1.3 Personnel. An approved agency shall employ experienced personnel educated in conducting, supervising and evaluating tests and/or inspections.

1703A.2 Written approval. Any material, appliance, equipment, system or method of construction meeting the requirements of this code shall be approved in writing after satisfactory completion of the required tests and submission of required test reports.

1703A.3 Approved record. For any material, appliance, equipment, system or method of construction that has been approved, a record of such approval, including the conditions and limitations of the approval, shall be kept on file in the building official's office and shall be open to public inspection at appropriate times.

1703A.4 Performance. Specific information consisting of test reports conducted by an approved testing agency in accordance with standards referenced in Chapter 35, or other such information as necessary, shall be provided for the building official to determine that the material meets the applicable code requirements.

1703A.4.1 Research and investigation. Sufficient technical data shall be submitted to the building official to substantiate the proposed use of any material or assembly. If it is determined that the evidence submitted is satisfactory proof of performance for the use intended, the building official shall approve the use of the material or assembly subject to the requirements of this code. The costs, reports and investigations required under these provisions shall be paid by the permit applicant.

1703A.4.2 Research reports. Supporting data, where necessary to assist in the approval of materials or assemblies not specifically provided for in this code, shall consist of valid research reports from approved sources.

1703A.5 Labeling. Where materials or assemblies are required by this code to be labeled, such materials and assemblies shall be labeled by an approved agency in accordance with Section 1703A. Products and materials required to be labeled shall be labeled in accordance with the procedures set forth in Sections 1703A.5.1 through 1703A.5.3.

1703A.5.1 Testing. An approved agency shall test a representative sample of the product or material being labeled to the relevant standard or standards. The approved agency shall maintain a record of the tests performed. The record shall provide sufficient detail to verify compliance with the test standard.

1703A.5.2 Inspection and identification. The approved agency shall periodically perform an inspection, which shall be in-plant if necessary, of the product or material that is to be labeled. The inspection shall verify that the labeled product or material is representative of the product or material tested.

1703A.5.3 Label information. The label shall contain the manufacturer's or distributor's identification, model number, serial number or definitive information describing the product or material's performance characteristics and approved agency's identification.

1703A.6 Heretofore approved materials. The use of any material already fabricated or of any construction already erected, which conformed to requirements or approvals heretofore in effect, shall be permitted to continue, if not detrimental to life, health or safety to the public.

1703A.7 Evaluation and follow-up inspection services. Where structural components or other items regulated by this code are not visible for inspection after completion of a prefabricated assembly, the permit applicant shall submit a

report of each prefabricated assembly. The report shall indicate the complete details of the assembly, including a description of the assembly and its components, the basis upon which the assembly is being evaluated, test results and similar information and other data as necessary for the building official to determine conformance to this code. Such a report shall be approved by the building official.

1703A.7.1 Follow-up inspection. The permit applicant shall provide for special inspections of fabricated items in accordance with Section 1704A.2.

1703A.7.2 Test and inspection records. Copies of necessary test and inspection records shall be filed with the building official.

SECTION 1704A - SPECIAL INSPECTIONS

1704A.1 General. Where application is made for construction as described in this section, the owner ~~or the registered design professional in responsible charge acting as the owner's agent~~ shall employ one or more special inspectors to provide inspections during construction on the types of work listed under Section 1704A. The special inspector shall be a qualified person who shall demonstrate competence, to the satisfaction of the building official, for inspection of the particular type of construction or operation requiring special inspection. These inspections are in addition to the inspections specified in Section 109, Appendix Chapter 1.

Exceptions:

1. Special inspections are not required for work of a minor nature or as warranted by conditions in the jurisdiction as approved by the building official.
2. Special inspections are not required for building components unless the design involves the practice of professional engineering or architecture as defined by applicable state statutes and regulations governing the professional registration and certification of engineers or architects.
3. Unless otherwise required by the building official, special inspections are not required for occupancies in Group R-3 as applicable in Section 101.2, Appendix Chapter 1 and occupancies in Group U that are accessory to a residential occupancy including, but not limited to, those listed in Section 312.1.

1704A.1.1 Statement of special inspections. The permit applicant shall submit a statement of special inspections prepared by the registered design professional in responsible charge in accordance with Section 106.1, Appendix Chapter 1 as a condition for permit issuance. This statement shall be in accordance with Section 1705A.

Exceptions:

1. Not permitted by OSHPD and DSA-SS. ~~A statement of special inspections is not required for structures designed and constructed in accordance with the conventional construction provisions of Section 2308.~~
2. The statement of special inspections is permitted to be prepared by a qualified person approved by the building official for construction not designed by a registered design professional.

1704A.1.2 Report requirement. *(Relocated from 1701A.3.2. CBC 2001)* The inspector(s) of record and Special special inspectors shall keep records of inspections. The inspector of record and special inspector shall furnish inspection reports to the building official and to the registered design professional in responsible charge *as required by Title 24, Part 1*. Reports shall indicate that work inspected was done in conformance to approved construction documents *as required by Title 24 Parts 1 and 2*. Discrepancies shall be brought to the immediate attention of the contractor for correction. If the discrepancies are not corrected, the discrepancies shall be brought to the attention of the building official and to the registered design professional in responsible charge prior to the completion of that phase of the work. A final report documenting required special inspections and correction of any discrepancies noted in the inspections shall be submitted at a point in time agreed upon by the permit applicant and the building official prior to the start of work.

Exception: *[DSA-SS] The term "inspector of record" is synonymous with "project inspector".*

1704A.2 Inspection of fabricators. Where fabrication of structural load-bearing members and assemblies is being performed on the premises of a fabricator's shop, special inspection of the fabricated items shall be required by this section and as required elsewhere in this code.

1704A.2.1 Fabrication and implementation procedures. The special inspector shall verify that the fabricator maintains detailed fabrication and quality control procedures that provide a basis for inspection control of the workmanship and the fabricator's ability to conform to approved construction documents and referenced standards. The special inspector shall review the procedures for completeness and adequacy relative to the code requirements for the fabricator's scope of work.

Exception: Special inspections as required by Section 1704A.2 shall not be required where the fabricator is approved in accordance with Section 1704A.2.2 *except as required by Sections 1704A.3, 1704A.4 and 1704A.6.*

1704A.2.2 Fabricator approval. Special inspections required by this code are not required where the work is done on the premises of a fabricator registered and approved to perform such work without special inspection. Approval shall be based upon review of the fabricator's written procedural and quality control manuals and periodic auditing of fabrication practices by an approved special inspection agency. At completion of fabrication, the approved fabricator shall submit a certificate of compliance to the building official stating that the work was performed in accordance with the approved construction documents.

1704A.3 Steel construction. The special inspections for steel elements of buildings and structures shall be as required by Section 1704A.3 and Table 1704A.3.

Exceptions:

1. Special inspection of the steel fabrication process shall not be required where the fabricator does not perform any welding, thermal cutting or heating operation of any kind as part of the fabrication process. In such cases, the fabricator shall be required to submit a detailed procedure for material control that demonstrates the fabricator's ability to maintain suitable records and procedures such that, at any time during the fabrication process, the material specification, grade and mill test reports for the main stress-carrying elements are capable of being determined.
2. The special inspector need not be continuously present during welding of the following items, provided the materials, welding procedures and qualifications of welders are verified prior to the start of the work; periodic inspections are made of the work in progress; and a visual inspection of all welds is made prior to completion or prior to shipment of shop welding.
 - 2.1. Single-pass fillet welds not exceeding $\frac{5}{16}$ inch (7.9 mm) in size.
 - 2.2. Floor and roof deck welding.
 - 2.3. Welded studs when used for structural diaphragm.
 - 2.4. Welded sheet steel for cold-formed steel framing members such as studs and joists.
 - 2.5. Welding of stairs and railing systems.

TABLE 1704A.3 - REQUIRED VERIFICATION AND INSPECTION OF STEEL CONSTRUCTION

VERIFICATION AND INSPECTION	CONTINUOUS	PERIODIC	REFERENCED STANDARD ^a	IBC CBC REFERENCE
1. Material verification of high-strength bolts, nuts and washers:				
a. Identification markings to conform to ASTM standards specified in the approved construction documents.	—	X	Applicable ASTM material specifications; AISC 360, Section A3.3	—
b. Manufacturer's certificate of compliance required.	—	X	—	—
2. Inspection of high-strength bolting:				
a. Bearing-type connections.	—	X	AISC 360, Section M2.5	1704A.3.3
b. Slip-critical connections.	X	X		
3. Material verification of structural steel:				
a. Identification markings to conform to ASTM standards specified in the approved construction documents.	—	—	ASTM A 6 or ASTM A 568	1708A.4
b. Manufacturers' certified mill test reports.	—	—	ASTM A 6 or ASTM A 568	
4. Material verification of weld filler materials:				
a. Identification markings to conform to AWS specification in the approved construction documents.	—	—	AISC 360, Section A3.5	—
b. Manufacturer's certificate of compliance required.	—	—	—	—
5. Inspection of welding:				
a. Structural steel:				
1) Complete and partial penetration groove welds.	X	—	AWS D1.1	1704A.3.1
2) Multipass fillet welds.	X	—		
3) Single-pass fillet welds $> \frac{3}{16}$ "	X	—		
4) Single-pass fillet welds $\leq \frac{3}{16}$ "	—	X		
5) Floor and roof deck welds.	—	X	AWS D1.3	—
b. Reinforcing steel:				
1) Verification of weldability of reinforcing steel other than ASTM A 706.	—	X	AWS D1.4 ACI 318: 3.5.2	—

2) Reinforcing steel-resisting flexural and axial forces in intermediate and special moment frames, and boundary elements of special reinforced concrete shear walls and shear reinforcement.	X	—		
3) Shear reinforcement.	X	—		
4) Other reinforcing steel.	—	X		
6. Inspection of steel frame joint details for compliance with approved construction documents:		X	—	1704A.3.2
a. Details such as bracing and stiffening.	—	—		
b. Member locations.	—	—		
c. Application of joint details at each connection.	—	—		

For SI: 1 inch = 25.4 mm.

a. Where applicable, see also Section 1707A.1, Special inspection for seismic resistance.

1704A.3.1 Welding. Welding inspection shall be in compliance with AWS D1.1. The basis for welding inspector qualification shall be AWS D1.1.

1704A.3.1.1 (Relocated from 2231A.5, CBC 2001) **Inspection of Welding.** Inspection of all shop and field welding operations, including the installation of automatic end-welded stud shear connectors shall be made by a qualified welding inspector approved by the enforcement agency. Such inspector shall be a person trained and thoroughly experienced in inspecting welding operations. The inspector's ability to distinguish between sound and unsound welding shall be reliably established. The minimum requirements for a qualified welding inspector shall be as those for an AWS certified welding inspector (CWI), as defined in the provisions of the AWS QC1-1996, Standard for AWS Certification of Welding Inspectors published by the American Welding Society. All welding inspectors shall be as approved by the enforcement agency.

The ability of each welder to produce sound welds of all types required by the work shall be established by welder qualification satisfactory to the enforcement agency.

Welding inspection of structural welding shall conform to the requirements of AWS D1.1 Structural Welding Code—Steel, 1998 edition, published by the American Welding Society, except as modified by this section.

Welding inspection of cold-formed steel members shall conform to the requirements of AWS D1.3.

The welding inspector shall make a systematic record of all welds. This record shall include in addition to other required records:

1. Identification marks of welders.
2. List of defective welds.

3. Manner of correction of defects.

The welding inspector shall check the material, equipment, details of construction and procedure, as well as the welds. The inspector shall also check the ability of the welder. The inspector shall verify that the installation procedure for automatic end-welded stud shear connectors is in accordance with the requirements of AWS D1.1, ~~Structural Welding Code. Steel, 1998 edition, published by the American Welding Society~~ and the approved plans and specifications. The inspector shall furnish the architect, structural engineer and the enforcement agency with a verified report that the welding is proper and has been done in conformity with AWS D1.1, ~~Structural Welding Code. Steel, 1998 edition, published by the American Welding Society~~ and the approved plans and specifications. The inspector shall use all means necessary to determine the quality of the weld. The inspector may use gamma ray, magnaflux, trepanning, sonics or any other aid to visual inspection which the inspector may deem necessary to be assured of the adequacy of the welding.

EXCEPTION: ~~Plant welding inspection of open web steel joists may be waived with the approval of the enforcement agency where welding inspection is provided at the jobsite.~~

1704A.3.2 Details. The special inspector shall perform an inspection of the steel frame to verify compliance with the details shown on the approved construction documents, such as bracing, stiffening, member locations and proper application of joint details at each connection.

1704A.3.2.1 (Relocated from 2231A.4, CBC 2001) **Inspection of Shop Fabrication.** Inspection of shop fabrication shall be required for significant structural detailed connection and fabrication work as directed by the enforcement agency. This inspection shall be made by a qualified inspector approved by the enforcement agency. The inspector shall furnish the architect, structural engineer and the enforcement agency with a report that the materials and workmanship conform to the approved plans and specifications.

~~[For OSHPD 1 & 4] When welds from web doubler plates or continuity plates occur in the k-area of rolled steel columns, the k-area adjacent to the welds shall be inspected after fabrication as required by the enforcement agency, using approved nondestructive methods conforming to AWS D1.1. The k-area is defined in wide flange shapes to be the area of the web immediately adjacent to the flange, extending from the fillet to a point approximately 1 1/2 inches beyond the point of tangency between the fillet and the web.~~

1704A.3.2.2 (Relocated from 2231A.5, CBC 2001) **Steel Joist and Joist Girder Inspection.** Special inspection is required during the manufacture and welding of steel joists or joist girders. The special inspector shall verify that proper quality control procedures and tests have been employed for all materials and the manufacturing process, and shall perform visual inspection of the finished product. The special inspector shall place a distinguishing mark, and/or tag with this distinguishing mark, on each inspected joist or joist girder. This mark or tag shall remain on the joist or joist girder throughout the job site receiving and erection process.

1704A.3.2.3 (Relocated from 2231A.5, CBC 2001) **Light-Framed Steel Truss Inspection.** The manufacture of cold-formed light framed steel trusses shall be continuously inspected by a qualified special inspector approved by the enforcement agency. The special inspector shall verify conformance of materials and manufacture with approved plans and specifications. The special inspector shall place a distinguishing mark, and/or tag with this distinguishing mark, on each inspected truss. This mark or tag shall remain on the truss throughout the job site receiving and erection process.

1704A.3.3 High-strength bolts. Installation of high-strength bolts shall be periodically inspected in accordance with AISC specifications.

1704A.3.3.1 General. While the work is in progress, the special inspector shall determine that the requirements for bolts, nuts, washers and paint; bolted parts and installation and tightening in such standards are met. For bolts requiring pretensioning, the special inspector shall observe the preinstallation testing and calibration procedures when such procedures are required by the installation method or by project plans or specifications; determine that all plies of connected materials have been drawn together and properly snugged and monitor the installation of bolts to verify that the selected procedure for installation is properly used to tighten bolts. For joints required to be tightened only to the snug-tight condition, the special inspector need only verify that the connected materials have been drawn together and properly snugged.

1704A.3.3.2 Periodic monitoring. Monitoring of bolt installation for pretensioning is permitted to be performed on a periodic basis when using the turn-of-nut method with matchmarking techniques, the direct tension indicator method or the alternate design fastener (twist-off bolt) method. Joints designated as snug tight need be inspected only on a periodic basis.

1704A.3.3.3 Continuous monitoring. Monitoring of bolt installation for pretensioning using the calibrated wrench method or the turn-of-nut method without matchmarking shall be performed on a continuous basis.

1704A.4 Concrete construction. The special inspections and verifications for concrete construction shall be as required by this section and Table 1704A.4.

Exception: Not permitted by OSHPD and DSA-SS. ~~Special inspections shall not be required for:-~~

- ~~1. Isolated spread concrete footings of buildings three stories or less in height that are fully supported on earth or rock.~~
- ~~2. Continuous concrete footings supporting walls of buildings three stories or less in height that are fully supported on earth or rock where:~~
 - ~~2.1. The footings support walls of light frame construction;~~
 - ~~2.2. The footings are designed in accordance with Table 1805.4.2; or~~
 - ~~2.3. The structural design of the footing is based on a specified compressive strength, f_c , no greater than 2,500 pounds per square inch (psi) (17.2 MPa), regardless of the compressive strength specified in the construction documents or used in the footing construction.~~
- ~~3. Nonstructural concrete slabs supported directly on the ground, including prestressed slabs on grade, where the effective prestress in the concrete is less than 150 psi (1.03 MPa).~~
- ~~4. Concrete foundation walls constructed in accordance with Table 1805.5(5).~~
- ~~5. Concrete patios, driveways and sidewalks, on grade.~~

TABLE 1704A.4 - REQUIRED VERIFICATION AND INSPECTION OF CONCRETE CONSTRUCTION

VERIFICATION AND INSPECTION	CONTINUOUS	PERIODIC	REFERENCED STANDARD ^a	IBC <u>CBC</u> REFERENCE
1. Inspection of reinforcing steel, including prestressing tendons, and placement.	—	X	ACI 318: 3.5, 7.1-7.7	1913 <u>A.4</u>
2. Inspection of reinforcing steel welding in accordance with Table 1704 <u>A.3</u> , Item 5b.	—	—	AWS D1.4 ACI 318: 3.5.2	—

3. Inspect bolts to be installed in concrete prior to and during placement of concrete where allowable loads have been increased.	X	—	—	1911A.5
4. Verifying use of required design mix.	—	X	ACI 318: Ch. 4, 5.2-5.4	1904A.2.2, 1913A.2, 1913A.3
5. At the time fresh concrete is sampled to fabricate specimens for strength tests, perform slump and air content tests, and determine the temperature of the concrete.	X	—	ASTM C 172 ASTM C 31 ACI 318: 5.6, 5.8	1913A.10
6. Inspection of concrete and shotcrete placement for proper application techniques.	X	—	ACI 318: 5.9, 5.10	1913A.6, 1913A.7, 1913A.8
7. Inspection for maintenance of specified curing temperature and techniques.	—	X	ACI 318: 5.11-5.13	1913A.9
8. Inspection of prestressed concrete: a. Application of prestressing forces. b. Grouting of bonded prestressing tendons in the seismic-force-resisting system.	X X	—	ACI 318: 18.20 ACI 318: 18.18.4	—
9. Erection of precast concrete members.	—	X	ACI 318: Ch. 16	—
10. Verification of in-situ concrete strength, prior to stressing of tendons in posttensioned concrete and prior to removal of shores and forms from beams and structural slabs.	—	X	ACI 318: 6.2	—
11. Inspect formwork for shape, location and dimensions of the concrete member being formed.	—	X	ACI 318: 6.1.1	—
<u>12. (Relocated from 1701A. 5, Item #18, CBC 2001) Post-Installed Anchors.</u>	<u>X</u>	<u>=</u>	<u>=</u>	<u>=</u>

For SI: 1 inch = 25.4 mm.

a. Where applicable, see also Section 1707A.1, Special inspection for seismic resistance.

1704A.4.1 Materials. In the absence of sufficient data or documentation providing evidence of conformance to quality standards for materials in Chapter 3 of ACI 318, the building official shall require testing of materials in accordance with the appropriate standards and criteria for the material in Chapter 3 of ACI 318. Weldability of reinforcement, except that which conforms to ASTM A 706, shall be determined in accordance with the requirements of Section 3.5.2 of ACI 318.

1704A.4.2 (Relocated from 1929A.12, CBC 2001) **Inspection of Welded Reinforcing Bars.** *Inspection of all shop and field structural welding operations shall be made by a qualified welding inspector approved by the enforcement agency. Such inspector shall be trained and thoroughly experienced in inspecting reinforcing bar welding operations. The inspector's ability to distinguish between sound and unsound welding shall be reliably established.*

The welding inspector shall make a systematic record of all welds. This record shall include:

1. Identification marks of welders.
2. List of defective welds.
3. Manner of correction of defects.

The welding inspector shall check the material, equipment, details of construction, and procedures as well as the welds. The inspector shall also check the ability of the welder. The welding inspector shall furnish the architect, structural engineer and the enforcement agency with a verified report that the welding which is required to be inspected is proper and has been done in conformity with the approved plans and specifications. The welding inspector shall use all means necessary to determine the quality of the weld. The inspector may use gamma ray, magnaflux, trepanning, sonics or any other aid to visual inspection which the inspector may deem necessary to assure the adequacy of the welding.

1704A.4.3 (Relocated from 1929A.4, CBC 2001) **Batch Plant Inspection.** Except as provided under Section 1929A.5 **1704A.4.4**, the quality and quantity of materials used in transit-mixed concrete and in batched aggregates shall be continuously inspected at the location where materials are measured by an approved special inspector.

1704A.4.4 (Relocated from 1929A.5, CBC 2001) **Waiver of Batch Plant Inspection.** Batch plant inspection may be waived under either of the following conditions:

1. The concrete plant complies fully with the requirements of ASTM C 94, Sections 8 and 9, and has a current certificate from the National Ready Mixed Concrete Association or another agency acceptable to the enforcement agency. The certification shall indicate that the plant has automatic batching and recording capabilities.
2. For one-story wood-frame or one-story light-steel buildings and isolated mat-type foundations supporting equipment only, where the specified compressive strength f_c of the concrete delivered to the jobsite is 3,500 psi (24.13 MPa) and where the f_c used in design is not greater than 2,500 psi (17.24 MPa).

When batch plant inspection is waived, the following requirements shall apply and shall be described in the contract specifications:

Approved inspector of the testing laboratory shall check the first batching at the start of work and furnish mix proportions to the licensed weighmaster.

Licensed weighmaster to positively identify materials as to quantity and certify to each load by a ticket.

Tickets shall be transmitted to the ~~project inspector~~ inspector of record by a truck driver with load identified thereon. Inspector will not accept the load without a load ticket identifying the mix and will keep a daily record of placements, identifying each truck, its load and time of receipt, and approximate location of deposit in the structure and will transmit a copy of the daily record to the enforcement agency.

Exception: [DSA-SS] The term "inspector of record" is synonymous with "project inspector".

At the end of the project, the weighmaster shall furnish an affidavit to the enforcement agency on Form SSS-411-8 certifying that all concrete furnished conforms in every particular to proportions established by mix designs.

1704A.4.5 (Relocated from 1929A.9, CBC 2001) **Inspection of Prestressed Concrete.**

1704A.4.5.1 In addition to the general inspection required for concrete work, all plant fabrication of prestressed concrete members or tensioning of posttensioned members constructed at the site shall be continuously inspected by an inspector specially approved for this purpose by the enforcement agency.

1704A.4.5.2 To be eligible for approval, the inspector shall be examined as to his or her knowledge and experience in prestressed concrete construction.

1704A.4.5.3 The prestressed concrete plant fabrication inspector shall check the materials, equipment, tensioning procedure and construction of the prestressed members. The inspector shall make a verified report identifying the members by mark and shall include such pertinent data as lot numbers of tendons used, tendon jacking forces, age and strength of concrete at time of tendon release and such other information that may be required.

1704A.4.5.4 The inspector of prestressed members posttensioned at the site shall check the condition of the prestressing tendons, anchorage assemblies and concrete in the area of the anchorage, the tensioning equipment and the tensioning procedure. The inspector shall make a verified report of the prestressing operation identifying the members or tendons by mark and including such pertinent data as the initial cable slack, net elongation of tendons, jacking force developed, and such other information as may be required.

1704A.4.5.5 The verified reports of construction shall show that of the inspector's own personal knowledge, the work covered by the report has been performed and materials used and installed in every material respect in compliance with the duly approved plans and specifications for plant fabrication inspection. The verified report shall be accompanied by test reports required for materials used. For site posttensioning inspections the verified report shall be accompanied by copies of calibration charts, certified by an approved testing laboratory, showing the relationship between gage readings and force applied by the jacks used in the prestressing procedure

1704A.4.6 (Relocated from 1905A.7.1, Item #8, CBC 2001) **Concrete Pre-Placement Inspection.** Concrete shall not be placed until the forms and reinforcement have been inspected, all preparations for the placement have been completed, and the preparations have been checked by the inspector of Record and Special Inspector, all subject to the observation of the structural engineer or architect.

1704A.4.7 (Relocated from 1929A.7, CBC 2001) **Placing Record.** A record shall be kept on the site of the time and date of placing the concrete in each portion of the structure. Such record shall be kept until the completion of the structure and shall be open to the inspection of the enforcement agency.

1704A.5 Masonry construction. Masonry construction shall be inspected and evaluated in accordance with the requirements of Sections 1704A.5.1 through 1704A.5.3, depending on the classification of the building or structure or nature of the occupancy, as defined by this code.

Exception: Not permitted by OSHPD and DSA-SS. Special inspections shall not be required for:

1. Empirically designed masonry, glass unit masonry or masonry veneer designed by Section 2109, 2110 or Chapter 14, respectively, or by Chapter 5, 7 or 6 of ACI 530/ASCE 5/TMS 402, respectively, when they are part of structures classified as Occupancy Category I, II or III in accordance with Section 1604.5.
2. Masonry foundation walls constructed in accordance with Table 1805.5(1), 1805.5(2), 1805.5(3) or 1805.5(4).
3. Masonry fireplaces, masonry heaters or masonry chimneys installed or constructed in accordance with Section 2111, 2112 or 2113, respectively.

1704A.5.1 Empirically designed masonry, glass **Glass** unit masonry and masonry veneer in Occupancy Category **II, III or IV**. The minimum special inspection program for empirically designed masonry, glass unit

masonry or masonry veneer designed by Section 2109, 2110 or Chapter 21A or 14, respectively, or by Chapter 5, 7 or 6 of ACI 530/ASCE 5/TMS 402, respectively, in structures classified as Occupancy Category II, III or IV, in accordance with Section 1604A.5, shall comply with Table 1704A.5.1.

TABLE 1704A.5.1 - LEVEL 1 SPECIAL INSPECTION

INSPECTION TASK	FREQUENCY OF INSPECTION		REFERENCE FOR CRITERIA		
	Continuous during task listed	Periodically during task listed	IBC CBC section	ACI 530/ASCE 5/TMS 402 ^a	ACI 530.1/ASCE 6/TMS 602 ^a
1. As masonry construction begins, the following shall be verified to ensure compliance:					
a. Proportions of site-prepared mortar.	—	X	—	—	Art. 2.6A
b. Construction of mortar joints.	—	X	—	—	Art. 3.3B
c. Location of reinforcement, connectors, prestressing tendons and anchorages.	—	X	—	—	Art. 3.4, 3.6A
d. Prestressing technique.	—	X	—	—	Art. 3.6B
e. Grade and size of prestressing tendons and anchorages.	—	X	—	—	Art. 2.4B, 2.4H
2. The inspection program shall verify:					
a. Size and location of structural elements.	—	X	—	—	Art. 3.3G
b. Type, size and location of anchors, including other details of anchorage of masonry to structural members, frames or other construction.	—	X	—	Sec. 1.2.2(e), 2.1.4, 3.1.6	—
c. Specified size, grade and type of reinforcement.	—	X	—	Sec. 1.13	Art. 2.4, 3.4
d. Welding of reinforcing bars.	X	—	—	Sec. 2.1.10.7.2, 3.3.3.4(b)	—
e. Protection of masonry during cold weather (temperature below 40°F) or hot weather (temperature above 90°F).	—	X	Sec. 2104A.3, 2104A.4	—	Art. 1.8C, 1.8D
f. Application and measurement of prestressing force.	—	X	—	—	Art. 3.6B
3. Prior to grouting, the following shall be verified to ensure compliance:					
a. Grout space is clean.	—	X	—	—	Art. 3.2D
b. Placement of reinforcement and connectors and prestressing tendons and anchorages.	—	X	—	Sec. 1.13	Art. 3.4
c. Proportions of site-prepared grout and prestressing grout for bonded tendons.	—	X	—	—	Art. 2.6B
d. Construction of mortar joints.	—	X	—	—	Art. 3.3B

4. Grout placement shall be verified to ensure compliance with code and construction document provisions.	X	—	—	—	Art 3.5
a. Grouting of prestressing bonded tendons.	X	—	—	—	Art. 3.6C
5. Preparation of any required grout specimens, mortar specimens and/or prisms shall be observed.	X	—	Sec. 2105A.2.2 2105A.3	—	Art. 1.4
6. Compliance with required inspection provisions of the construction documents and the approved submittals shall be verified.	—	X	—	—	Art. 1.5
<u>7. (Relocated from 1701A.5 Item #18, CBC 2001) Post-Installed Anchors.</u>	<u>X</u>	<u>=</u>	<u>=</u>	<u>=</u>	<u>=</u>

For SI: °C = (°F - 32)/1.8.

a. The specific standards referenced are those listed in Chapter 35.

1704A.5.2 Engineered masonry in Occupancy Category I, ~~II or III~~. The minimum special inspection program for masonry designed by Section 2107A or 2108A or by chapters other than ~~Chapters~~ Chapter 5, 6 or 7 of ACI 530/ASCE 5/TMS 402 in structures classified as Occupancy Category I, ~~II or III~~, in accordance with Section 1604A.5, shall comply with Table 1704A.5.1.

1704A.5.3 Engineered masonry in Occupancy Category II, III or IV. The minimum special inspection program for masonry designed by Section 2107A or 2108A or by chapters other than ~~Chapters~~ Chapter 5, 6 or 7 of ACI 530/ASCE 5/TMS 402 in structures classified as Occupancy Category II, III or IV, in accordance with Section 1604A.5, shall comply with Table 1704A.5.3.

TABLE 1704A.5.3 - LEVEL 2 SPECIAL INSPECTION

INSPECTION TASK	FREQUENCY OF INSPECTION		REFERENCE FOR CRITERIA		
	Continuous during task listed	Periodically during task listed	IBC <u>CBC</u> section	ACI 530/ASCE 5/TMS 402 ^a	ACI 530.1/ASCE 6/TMS 602 ^a
1. From the beginning of masonry construction, the following shall be verified to ensure compliance:					
a. Proportions of site-prepared mortar, grout and prestressing grout for bonded tendons.	—	X	—	—	Art. 2.6A
b. Placement of masonry units and construction of mortar joints.	—	X	—	—	Art. 3.3B
c. Placement of reinforcement, connectors and prestressing tendons and anchorages.	—	X	—	Sec. 1.13	Art. 3.4, 3.6A
d. Grout space prior to grouting.	X	—	—	—	Art. 3.2D
e. Placement of grout.	X	—	—	—	Art. 3.5

f. Placement of prestressing grout.	X	—	—	—	Art. 3.6C
2. The inspection program shall verify:					
a. Size and location of structural elements.	—	X	—	—	Art. 3.3G
b. Type, size and location of anchors, including other details of anchorage of masonry to structural members, frames or other construction.	X	—	—	Sec. 1.2.2(e), 2.1.4, 3.1.6	—
c. Specified size, grade and type of reinforcement.		X	—	Sec. 1.13	Art. 2.4, 3.4
d. Welding of reinforcing bars.	X	—	—	Sec. 2.1.10.7.2, 3.3.3.4(b)	—
e. Protection of masonry during cold weather (temperature below 40°F) or hot weather (temperature above 90°F).	—	X	Sec. 2104A.3, 2104A.4	—	Art. 1.8C, 1.8D
f. Application and measurement of prestressing force.	X	—	—	—	Art. 3.6B
3. Preparation of any required grout specimens, mortar specimens and/or prisms shall be observed.	X	—	Sec. 2105A.2.2, 2105A.3	—	Art. 1.4
4. Compliance with required inspection provisions of the construction documents and the approved submittals shall be verified.	—	X	—	—	Art. 1.5
5. (Relocated from 1701A.5 Item #18, CBC 2001) <u>Post-Installed Anchors.</u>	X	—	—	—	—

For SI: °C = (°F - 32)/1.8.

- a. The specific standards referenced are those listed in Chapter 35.

1704A.6 Wood construction. Special inspections of the fabrication process of prefabricated wood structural elements and assemblies shall be in accordance with Section 1704A.2 except as modified in this section. Special inspections of site and shop built assemblies shall be in accordance with this section.

1704A.6.1 High-load diaphragms. High-load diaphragms designed in accordance with Table 2306.3.2 shall be installed with special inspections as indicated in Section 1704A.1. The special inspector shall inspect the wood structural panel sheathing to ascertain whether it is of the grade and thickness shown on the approved building plans. Additionally, the special inspector must verify the nominal size of framing members at adjoining panel edges, the nail or staple diameter and length, the number of fastener lines and that the spacing between fasteners in each line and at edge margins agrees with the approved building plans.

1704A.6.2 Wood Structural Elements and Assemblies. Special inspection of wood structural elements and assemblies is required, as specified in this section, to ensure conformance with approved drawings and specifications, and applicable standards

The special inspector shall furnish a verified report to the design professional in general responsible charge of construction observation, the structural engineer, and the enforcement agency.

in accordance with Title 24, Part 1 and this chapter. The verified report shall list all inspected members or trusses, and shall indicate whether or not the inspected members or trusses conform with applicable standards and the approved drawings and specifications. Any non-conforming items shall be indicated on the verified report.

1704A.6.2.1 (Relocated from 2337A.1, CBC 2001) **Structural Glued- Laminated Timber.** Manufacture of all structural glued laminated timber shall be continuously inspected by a qualified special inspector approved by the enforcement agency.

The special inspector shall verify that proper quality control procedures and tests have been employed for all materials and the manufacturing process, and shall perform visual inspection of the finished product. Each inspected member shall be stamped by the special inspector with an identification mark.

Exception: Special Inspection is not required for non-custom members of 5-1/8 inch maximum width and 18 inch maximum depth, and with a maximum clear span of 32 feet, manufactured and marked in accordance with ANSI/AITC A 190.1 Section 6.1.1 for non-custom members.

1704A.6.2.2 (Relocated from 2337A.3, CBC 2001) **Manufactured open web trusses.** The manufacture of open web trusses shall be continuously inspected by a qualified special inspector approved by the enforcement agency.

The special inspector shall verify that proper quality control procedures and tests have been employed for all materials and the manufacturing process, and shall perform visual inspection of the finished product. Each inspected truss shall be stamped with an identification mark by the special inspector.

1704A.6.3 (Relocated from 2337A.2, CBC 2001) **Timber Connectors.** The installation of all timber connectors shall be continuously inspected by a qualified inspector approved by the enforcement agency. The inspector shall furnish the architect, structural engineer and the enforcement agency with a report duly verified by him that the materials, timber connectors and workmanship conform to the approved plans and specifications.

1704A.7 Soils. Special inspections for existing site soil conditions, fill placement and load-bearing requirements shall be as required by this section and Table 1704A.7. The approved soils report, required by Section 1802A.2, and the documents prepared by the registered design professional in responsible charge shall be used to determine compliance. During fill placement, the special inspector shall determine that proper materials and procedures are used in accordance with the provisions of the approved soils report, as specified in Section 1803A.5.

Exception: Special inspection is not required during placement of controlled fill having a total depth of 12 inches (305 mm) or less.

TABLE 1704A.7 - REQUIRED VERIFICATION AND INSPECTION OF SOILS

VERIFICATION AND INSPECTION TASK	CONTINUOUS DURING TASK LISTED	PERIODICALLY DURING TASK LISTED
1. Verify materials below footings are adequate to achieve the design bearing capacity.	—	X
2. Verify excavations are extended to proper depth and have reached proper material.	—	X
3. Perform classification and testing of controlled fill materials.	—	X

4. Verify use of proper materials, densities and lift thicknesses during placement and compaction of controlled fill.	X	—
5. Prior to placement of controlled fill, observe subgrade and verify that site has been prepared properly.	—	X

1704A.7.1 (Relocated from 3301 1, CBC 2001) **Soil Fill.** All fills used to support the foundations of any building or structure shall be placed under the direction of a geotechnical engineer, and the placement of the fill shall be inspected by the geotechnical engineer or his or her qualified representative. It shall be the responsibility of such geotechnical engineer to see that the procedures used in placing fills meet the requirements of the specifications and to coordinate all fill inspection and testing during the construction involving such fills.

The duties of the geotechnical engineer shall include, but need not be limited to, the observation of cleared areas and benches prepared to receive fill; observation of the removal of all unsuitable soils and other materials; the approval of soils to be used as fill material; the inspection of placement and compaction of fill materials; the testing of the completed fills; and the inspection or review of geotechnical drainage devices where required by the soils investigation, buttress fills or other similar protective measures.

A verified report shall be submitted to the enforcement agency by the geotechnical engineer. The report shall indicate that all the tests required by the plans and specifications were completed and that the tested materials were in compliance with the plans and specifications and the recommendations of the soils investigation report.

1704A.8 Pile foundations. Special inspections shall be performed during installation and testing of pile foundations as required by Table 1704A.8. The approved soils report, required by Section 1802A.2, and the documents prepared by the registered design professional in responsible charge shall be used to determine compliance.

TABLE 1704A.8 - REQUIRED VERIFICATION AND INSPECTION OF PILE FOUNDATIONS

VERIFICATION AND INSPECTION TASK	CONTINUOUS DURING TASK LISTED	PERIODICALLY DURING TASK LISTED
1. Verify pile materials, sizes and lengths comply with the requirements.	X	—
2. Determine capacities of test piles and conduct additional load tests, as required.	X	—
3. Observe driving operations and maintain complete and accurate records for each pile.	X	—
4. Verify placement locations and plumbness, confirm type and size of hammer, record number of blows per foot of penetration, determine required penetrations to achieve design capacity, record tip and butt elevations and document any pile damage.	X	—
5. For steel piles, perform additional inspections in accordance with Section 1704A.3.	—	—
6. For concrete piles and concrete-filled piles, perform additional inspections in accordance with Section 1704A.4.	—	—

7. For specialty piles, perform additional inspections as determined by the registered design professional in responsible charge.	—	—
8. For augered uncased piles and caisson piles, perform inspections in accordance with Section 1704A.9.	—	—

1704A.8.1 (Relocated from 1809A.6, CBC 2001) **Pile Observation.** The installation of piles shall be continuously observed by a qualified representative of the geotechnical engineer responsible for that portion of the project. The representative of the geotechnical engineer shall be examined by the enforcement agency to determine his / her knowledge and experience in pile-driving operations. The enforcement agency shall approve or reject the representative based on this examination and his / her qualification record.

The representative of the geotechnical engineer shall make a report of the pile-driving operation giving such pertinent data as the physical characteristics of the pile-driving equipment, identifying marks for each pile, the total depth of embedment for each pile; and when the allowable pile loads are determined by a dynamic load formula, the design formula used, and the permanent penetration under the last 10 blows. One copy of the report shall be sent to the enforcement agency.

1704A.9 Pier foundations. Special inspections shall be performed during installation and testing of pier foundations as required by Table 1704A.9. The approved soils report, required by Section 1802A.2, and the documents prepared by the registered design professional in responsible charge shall be used to determine compliance.

TABLE 1704A.9 - REQUIRED VERIFICATION AND INSPECTION OF PIER FOUNDATIONS

VERIFICATION AND INSPECTION TASK	CONTINUOUS DURING TASK LISTED	PERIODICALLY DURING TASK LISTED
1. Observe drilling operations and maintain complete and accurate records for each pier.	X	—
2. Verify placement locations and plumbness, confirm pier diameters, bell diameters (if applicable), lengths, embedment into bedrock (if applicable) and adequate end bearing strata capacity.	X	—
3. For concrete piers, perform additional inspections in accordance with Section 1704A.4.	—	—
4. For masonry piers, perform additional inspections in accordance with Section 1704A.5.	—	—

1704A.9.1 (Relocated from 1809A.7.1, CBC 2001) **Pier Observation.** The provisions of Section 1808A.2 shall apply to belled caissons. The belled base of each pier shall be inspected by a qualified representative of the geotechnical engineer to verify the bell size and foundation soil classification. The sloped sides of the belled bases shall be limited to a slope of 2 units vertical to 1 unit horizontal (200% slope) unless reinforced as for a concrete spread footing.

1704A.10 Sprayed fire-resistant materials. Special inspections for sprayed fire-resistant materials applied to structural elements and decks shall be in accordance with Sections 1704A.10.1 through 1704A.10.5. Special inspections shall be based on the fire-resistance design as designated in the approved construction documents.

1704A.10.1 Structural member surface conditions. The surfaces shall be prepared in accordance with the approved fire-resistance design and the approved manufacturer's written instructions. The prepared surface of structural members to be sprayed shall be inspected before the application of the sprayed fire-resistant material.

1704A.10.2 Application. The substrate shall have a minimum ambient temperature before and after application as specified in the approved manufacturer's written instructions. The area for application shall be ventilated during and after application as required by the approved manufacturer's written instructions.

1704A.10.3 Thickness. The average thickness of the sprayed fire-resistant materials applied to structural elements shall not be less than the thickness required by the approved fire-resistant design. Individually measured thickness, which exceeds the thickness specified in a design by $\frac{1}{4}$ inch (6.4 mm) or more, shall be recorded as the thickness specified in the design plus $\frac{1}{4}$ inch (6.4 mm). For design thicknesses 1 inch (25 mm) or greater, the minimum allowable individual thickness shall be the design thickness minus $\frac{1}{4}$ inch (6.4 mm). For design thicknesses less than 1 inch (25 mm), the minimum allowable individual thickness shall be the design thickness minus 25 percent. Thickness shall be determined in accordance with ASTM E 605. Samples of the sprayed fire-resistant materials shall be selected in accordance with Sections 1704A.10.3.1 and 1704A.10.3.2.

1704A.10.3.1 Floor, roof and wall assemblies. The thickness of the sprayed fire-resistant material applied to floor, roof and wall assemblies shall be determined in accordance with ASTM E 605 by taking the average of not less than four measurements for each 1,000 square feet (93 m²) of the sprayed area on each floor or part thereof.

1704A.10.3.2 Structural framing members. The thickness of the sprayed fire-resistant material applied to structural members shall be determined in accordance with ASTM E 605. Thickness testing shall be performed on not less than 25 percent of the structural members on each floor.

1704A.10.4 Density. The density of the sprayed fire-resistant material shall not be less than the density specified in the approved fire-resistant design. Density of the sprayed fire-resistant material shall be determined in accordance with ASTM E 605.

1704A.10.5 Bond strength. The cohesive/adhesive bond strength of the cured sprayed fire-resistant material applied to structural elements shall not be less than 150 pounds per square foot (psf) (7.18 kN/m²). The cohesive/adhesive bond strength shall be determined in accordance with the field test specified in ASTM E 736 by testing in-place samples of the sprayed fire-resistant material selected in accordance with Sections 1704A.10.5.1 and 1704A.10.5.2.

1704A.10.5.1 Floor, roof and wall assemblies. The test samples for determining the cohesive/adhesive bond strength of the sprayed fire-resistant materials shall be selected from each floor, roof and wall assembly at the rate of not less than one sample for every 10,000 square feet (929 m²) or part thereof of the sprayed area in each story.

1704A.10.5.2 Structural framing members. The test samples for determining the cohesive/adhesive bond strength of the sprayed fire-resistant materials shall be selected from beams, girders, joists, trusses and columns at the rate of not less than one sample for each type of structural framing member for each 10,000 square feet (929 m²) of floor area or part thereof in each story.

1704A.11 Mastic and intumescent fire-resistant coatings. Special inspections for mastic and intumescent fire-resistant coatings applied to structural elements and decks shall be in accordance with AWCI 12-B. Special inspections shall be based on the fire-resistance design as designated in the approved construction documents.

1704A.12 Exterior insulation and finish systems (EIFS). Special inspections shall be required for all EIFS applications.

Exceptions:

1. Special inspections shall not be required for EIFS applications installed over a water-resistive barrier with a means of draining moisture to the exterior.

2. Special inspections shall not be required for EIFS applications installed over masonry or concrete walls.

1704A.13 Special cases. Special inspections shall be required for proposed work that is, in the opinion of the building official, unusual in its nature, such as, but not limited to, the following examples:

1. Construction materials and systems that are alternatives to materials and systems prescribed by this code.
2. Unusual design applications of materials described in this code.
3. Materials and systems required to be installed in accordance with additional manufacturer's instructions that prescribe requirements not contained in this code or in standards referenced by this code.

[F] 1704A.14 Special inspection for smoke control. Smoke control systems shall be tested by a special inspector.

[F] 1704A.14.1 Testing scope. The test scope shall be as follows:

1. During erection of ductwork and prior to concealment for the purposes of leakage testing and recording of device location.
2. Prior to occupancy and after sufficient completion for the purposes of pressure difference testing, flow measurements and detection and control verification.

[F] 1704A.14.2 Qualifications. Special inspection agencies for smoke control shall have expertise in fire protection engineering, mechanical engineering and certification as air balancers.

1704A.15 (Relocated from 1929A.10, CBC 2001) **Inspection of Pneumatically Placed Concrete Work (Shotcrete).** All shotcrete work shall be continuously inspected during placing by an inspector specially approved for that purpose by the enforcement agency. The special shotcrete inspector shall check the materials, placing equipment, details of construction and construction procedure. The inspector shall furnish a verified report that of his or her own personal knowledge the work covered by the report has been performed and materials used and installed in every material respect in compliance with the duly approved plans and specifications.

1704A.15.1 (Relocated from 1924A.11.2, CBC 2001) **Visual examination for structural soundness of in-place shotcrete.** Completed shotcrete work shall be checked visually for reinforcing bar embedment, voids, rock pockets, sand streaks and similar deficiencies by examining a minimum of three 3-inch (76 mm) cores taken from three areas chosen by the design engineer which represent the worst congestion of reinforcing bars occurring in the project. Extra reinforcing bars may be added to noncongested areas and cores may be taken from these areas. The cores shall be examined by the special inspector and a report submitted to the enforcement agency prior to final approval of the shotcrete.

Exception: Shotcrete work fully supported on earth, minor repairs, and when, in the opinion of the enforcement agency, no special hazard exists.

1704A.16 (Relocated from 1701A.5, Item #8, CBC 2001) **Reinforced gypsum concrete.** All gypsum concrete work shall be continuously inspected when mixed and placed.

SECTION 1705A - STATEMENT OF SPECIAL INSPECTIONS

1705A.1 General. Where special inspection or testing is required by Section 1704A, 1707A or 1708A, the registered design professional in responsible charge shall prepare a statement of special inspections in accordance with Section 1705A for submittal by the permit applicant (see Section 1704A.1.1).

1705A.2 Content of statement of special inspections. The statement of special inspections shall identify the following:

1. The materials, systems, components and work required to have special inspection or testing by the building official or by the registered design professional responsible for each portion of the work.
2. The type and extent of each special inspection.
3. The type and extent of each test.
4. Additional requirements for special inspection or testing for seismic or wind resistance as specified in Section 1705A.3, 1705A.4, 1707A or 1708A.
5. For each type of special inspection, identification as to whether it will be continuous special inspection or periodic special inspection.

1705A.3 Seismic resistance. The statement of special inspections shall include seismic requirements for the following cases:

1. The seismic-force-resisting systems in structures assigned to Seismic Design Category C, D, E or F in accordance with Section 1613A.
2. Designated seismic systems in structures assigned to Seismic Design Category D, E or F.
3. The following additional systems and components in structures assigned to Seismic Design Category C:
 - 3.1. Heating, ventilating and air-conditioning (HVAC) ductwork containing hazardous materials and anchorage of such ductwork.
 - 3.2. Piping systems and mechanical units containing flammable, combustible or highly toxic materials.
 - 3.3. Anchorage of electrical equipment used for emergency or standby power systems.
4. The following additional systems and components in structures assigned to Seismic Design Category D:
 - 4.1. Systems required for Seismic Design Category C.
 - 4.2. Exterior wall panels and their anchorage.
 - 4.3. Suspended ceiling systems and their anchorage.
 - 4.4. Access floors and their anchorage.
 - 4.5. Steel storage racks and their anchorage, where the importance factor is equal to 1.5 in accordance with Section 15.5.3 of ASCE 7.
5. The following additional systems and components in structures assigned to Seismic Design Category E or F:
 - 5.1. Systems required for Seismic Design Categories C and D.
 - 5.2. Electrical equipment.

Exception: ~~Not permitted by OSHPD and DSA-SS.~~ Seismic requirements are permitted to be excluded from the statement of special inspections for structures designed and constructed in accordance with the following:

1. The structure consists of light frame construction; the design spectral response acceleration at short periods, S_{DS} , as determined in Section 1613A.5.4, does not exceed 0.5g; and the height of the structure does not exceed 35 feet (10 668 mm) above grade plane; or
2. The structure is constructed using a reinforced masonry structural system or reinforced concrete structural system; the design spectral response acceleration at short periods, S_{DS} , as determined in Section 1613A.5.4;

does not exceed 0.5g; and the height of the structure does not exceed 25 feet (7620 mm) above grade plane; or

~~3. Detached one- or two-family dwellings not exceeding two stories in height, provided the structure does not have any of the following plan or vertical irregularities in accordance with Section 12.3.2 of ASCE 7:~~

~~3.1. Torsional irregularity.~~

~~3.2. Nonparallel systems.~~

~~3.3. Stiffness irregularity-extreme soft story and soft story.~~

~~3.4. Discontinuity in capacity-weak story.~~

1705A.3.1 Seismic requirements in the statement of special inspections. When Section 1705A.3 specifies that seismic requirements be included, the statement of special inspections shall identify the following:

1. The designated seismic systems and seismic-force-resisting systems that are subject to special inspections in accordance with Section 1705A.3.
2. The additional special inspections and testing to be provided as required by Sections 1707A and 1708A and other applicable sections of this code, including the applicable standards referenced by this code.

1705A.4 Wind resistance. The statement of special inspections shall include wind requirements for structures constructed in the following areas:

1. In wind Exposure Category B, where the 3-second-gust basic wind speed is 120 miles per hour (mph) (52.8 m/s) or greater.
2. In wind Exposure Category C or D, where the 3-second-gust basic wind speed is 110 mph (49 m/s) or greater.

1705A.4.1 Wind requirements in the statement of special inspections. When Section 1705A.4 specifies that wind requirements be included, the statement of special inspections shall identify the main windforce-resisting systems and wind-resisting components subject to special inspections as specified in Section 1705A.4.2.

1705A.4.2 Detailed requirements. The statement of special inspections shall include at least the following systems and components:

1. Roof cladding and roof framing connections.
2. Wall connections to roof and floor diaphragms and framing.
3. Roof and floor diaphragm systems, including collectors, drag struts and boundary elements.
4. Vertical windforce-resisting systems, including braced frames, moment frames and shear walls.
5. Windforce-resisting system connections to the foundation.
6. Fabrication and installation of systems or components required to meet the impact-resistance requirements of Section 1609A.1.2.

Exception: Fabrication of manufactured systems or components that have a label indicating compliance with the wind-load and impact-resistance requirements of this code.

SECTION 1706A - CONTRACTOR RESPONSIBILITY

1706A.1 Contractor responsibility. Each contractor responsible for the construction of a main wind- or seismic-force-resisting system, designated seismic system or a wind- or seismic-resisting component listed in the statement of special inspections shall submit a written statement of responsibility to the building official and the owner prior to the commencement of work on the system or component. The contractor's statement of responsibility shall contain the following:

1. Acknowledgment of awareness of the special requirements contained in the statement of special inspections;
2. Acknowledgment that control will be exercised to obtain conformance with the construction documents approved by the building official;
3. Procedures for exercising control within the contractor's organization, the method and frequency of reporting and the distribution of the reports; and
4. Identification and qualifications of the person(s) exercising such control and their position(s) in the organization.

SECTION 1707A - SPECIAL INSPECTIONS FOR SEISMIC RESISTANCE

1707A.1 Special inspections for seismic resistance. Special inspections itemized in Sections 1707A.2 through 1707A.10, unless exempted by the exceptions of Section 1704A.1, are required for the following:

1. The seismic-force-resisting systems in structures assigned to Seismic Design Category C, D, E or F, as determined in Section 1613A.
2. Designated seismic systems in structures assigned to Seismic Design Category D, E or F.
3. Architectural, mechanical and electrical components in structures assigned to Seismic Design Category C, D, E or F that are required in Sections 1707A.7 and 1707A.8.

1707A.2 Structural steel. Continuous special inspection is required for structural welding in accordance with AISC 341.

Exceptions:

1. Single-pass fillet welds not exceeding $\frac{5}{16}$ inch (7.9 mm) in size.
2. Floor and roof deck welding.

1707A.3 Structural wood. Continuous special inspection is required during field gluing operations of elements of the seismic-force-resisting system. Periodic special inspection is required for nailing, bolting, anchoring and other fastening of components within the seismic-force-resisting system, including wood shear walls, wood diaphragms, drag struts, braces, shear panels and hold-downs.

Exception: ~~Not permitted by OSHPD and DSA-SS. Special inspection is not required for wood shear walls, shear panels and diaphragms, including nailing, bolting, anchoring and other fastening to other components of the seismic-force-resisting system, where the fastener spacing of the sheathing is more than 4 inches (102 mm) on center (o.c.).~~

1707A.4 Cold-formed steel framing. Periodic special inspections ~~is~~ **are** required during welding operations of elements of the seismic-force-resisting system. Periodic special inspection is required for screw attachment, bolting, anchoring and other fastening of components within the seismic-force-resisting system, including struts, braces, and hold-downs.

1707A.5 Pier foundations. Special inspection is required for pier foundations for buildings assigned to Seismic Design Category C, D, E or F in accordance with Section 1613A. Periodic special inspection is required during placement of reinforcement and continuous special inspection is required during placement of the concrete.

1707A.6 Storage racks and access floors. Periodic special inspection is required during the anchorage of access floors and storage racks 8 feet (2438 mm) or greater in height in structures assigned to Seismic Design Category D, E or F.

1707A.7 Architectural components. Periodic special inspection is required during the erection and fastening of exterior cladding, interior and exterior nonbearing walls and interior and exterior veneer in structures assigned to Seismic Design Category D, E or F.

Exceptions: Not permitted by OSHPD and DSA-SS.

- ~~1. Special inspection is not required for architectural components in structures 30 feet (9144 mm) or less in height.~~
- ~~2. Special inspection is not required for cladding and veneer weighing 5 psf (24.5 N/m²) or less.~~
- ~~3. Special inspection is not required for interior nonbearing walls weighing 15 psf (73.5 N/m²) or less.~~

1707A.8 Mechanical and electrical components. Special inspection for mechanical and electrical equipment shall be as follows:

1. Periodic special inspection is required during the anchorage of electrical equipment for emergency or standby power systems in structures assigned to Seismic Design Category C, D, E or F;
2. Periodic special inspection is required during the installation of anchorage of other electrical equipment in structures assigned to Seismic Design Category E or F;
3. Periodic special inspection is required during installation of piping systems intended to carry flammable, combustible or highly toxic contents and their associated mechanical units in structures assigned to Seismic Design Category C, D, E or F;
4. Periodic special inspection is required during the installation of HVAC ductwork that will contain hazardous materials in structures assigned to Seismic Design Category C, D, E or F; and
5. Periodic special inspection is required during the installation of vibration isolation systems in structures assigned to Seismic Design Category C, D, E or F where the construction documents require a nominal clearance of 0.25 inches (6.4 mm) or less between the equipment support frame and restraint.

1707A.9 Designated seismic system verifications. The special inspector shall examine designated seismic systems requiring seismic qualification in accordance with Section 1708A.5 and verify that the label, anchorage or mounting conforms to the certificate of compliance.

1707A.10 Seismic isolation system. Periodic special inspection is required during the fabrication and installation of isolator units and energy dissipation devices that are part of the seismic isolation system. *(Relocated from 1664A.3, CBC 2001)* Continuous special inspection is required for prototype and production testing of isolator units and energy dissipation devices that are part of the seismic isolation system.

SECTION 1708A - STRUCTURAL TESTING FOR SEISMIC RESISTANCE

1708A.1 Masonry. Testing and verification of masonry materials and assemblies prior to construction shall comply with the requirements of Sections 1708A.1.1 through 1708A.1.4, depending on the classification of the building or structure or nature of the occupancy, as defined by this code.

1708A.1.1 Empirically designed masonry and glass Glass unit masonry in Occupancy Category I, ~~H or III~~. For masonry designed by Section ~~2109 or 2110~~ 2110A or 2115A or by Chapter ~~5 or 7~~ of ACI 530/ASCE 5/TMS 402 in structures classified as Occupancy Category I, ~~H or III~~, in accordance with Section 1604A.5, certificates of compliance used in masonry construction shall be verified prior to construction.

1708A.1.2 Empirically designed masonry and glass Glass unit masonry in Occupancy Category II, III or IV. The minimum testing and verification prior to construction for masonry designed by Section ~~2109 or 2110~~ 2110A or 2115A or by Chapter ~~5 or 7~~ of ACI 530/ASCE 5/TMS 402 in structures classified as Occupancy Category II, III or IV, in accordance with Section 1604A.5, shall comply with the requirements of Table 1708A.1.2.

TABLE 1708A.1.2 - LEVEL 1 QUALITY ASSURANCE

MINIMUM TESTS AND SUBMITTALS
Certificates of compliance used in masonry construction.
Verification of f'_m and F_{AAC} prior to construction, except where specifically exempted by this code.

1708A.1.3 Engineered masonry in Occupancy Category I, ~~H or III~~. The minimum testing and verification prior to construction for masonry designed by Section 2107A or 2108A or by chapters other than Chapter 5, 6 or 7 of ACI 530/ASCE 5/TMS 402 in structures classified as Occupancy Category I, ~~H or III~~, in accordance with Section 1604A.5, shall comply with Table 1708A.1.2.

1708A.1.4 Engineered masonry in Occupancy Category II, III or IV. The minimum testing and verification prior to construction for masonry designed by Section 2107A or 2108A or by chapters other than Chapter 5, 6 or 7 of ACI 530/ASCE 5/TMS 402 in structures classified as Occupancy Category II, III or IV, in accordance with Section 1604A.5, shall comply with Table 1708A.1.4.

TABLE 1708A.1.4 - LEVEL 2 QUALITY ASSURANCE

MINIMUM TESTS AND SUBMITTALS
Certificates of compliance used in masonry construction.
Verification of f'_m and F_{AAC} prior to construction and every 5,000 square feet during construction.
Verification of proportions of materials in mortar and grout as delivered to the site.

For SI: 1 square foot = 0.0929
m².

1708A.2 Testing for seismic resistance. The tests specified in Sections 1708A.3 through 1708A.6 are required for the following:

1. The seismic-force-resisting systems in structures assigned to Seismic Design Category C, D, E or F, as determined in Section 1613A.

2. Designated seismic systems in structures assigned to Seismic Design Category D, E or F.
3. Architectural, mechanical and electrical components in structures assigned to Seismic Design Category C, D, E or F that are required in Section 1708A.5.

1708A.3 Reinforcing and prestressing steel. Certified mill test reports shall be provided for each shipment of reinforcing steel used to resist flexural, shear and axial forces in reinforced concrete intermediate frames, special moment frames and boundary elements of special reinforced concrete or reinforced masonry shear walls. Where ASTM A 615 reinforcing steel is used to resist earthquake-induced flexural and axial forces in special moment frames and in wall boundary elements of shear walls in structures assigned to Seismic Design Category D, E or F, as determined in Section 1613A, the testing requirements of ACI 318 shall be met. Where ASTM A 615 reinforcing steel is to be welded, chemical tests shall be performed to determine weldability in accordance with Section 3.5.2 of ACI 318.

1708A.4 Structural steel. The testing contained in the quality assurance plan shall be as required by AISC 341 and the additional requirements herein. The acceptance criteria for nondestructive testing shall be as required in AWS D1.1 as specified by the registered design professional.

Base metal thicker than 1.5 inches (38 mm), where subject to through-thickness weld shrinkage strains, shall be ultrasonically tested for discontinuities behind and adjacent to such welds after joint completion. Any material discontinuities shall be accepted or rejected on the basis of ASTM A 435 or ASTM A 898 (Level 1 criteria) and criteria as established by the registered design professional(s) in responsible charge and the construction documents.

1708A.5 Seismic qualification of mechanical and electrical equipment. The registered design professional in responsible charge shall state the applicable seismic qualification requirements for designated seismic systems on the construction documents. Each manufacturer of designated seismic system components shall test or analyze the component and its mounting system or anchorage and submit a certificate of compliance for review and acceptance by the registered design professional in responsible charge of the design of the designated seismic system and for approval by the building official. Qualification shall be by an actual test on a shake table, by three-dimensional shock tests, by an analytical method using dynamic characteristics and forces, by the use of experience data (i.e., historical data demonstrating acceptable seismic performance) or by a more rigorous analysis providing for equivalent safety.

1708A.6 Seismically isolated structures. For required system tests, see Section 17.8 of ASCE 7.

SECTION 1709A - STRUCTURAL OBSERVATIONS

1709A.1 General. Where required by the provisions of Section 1709A.2 or 1709A.3 the owner shall employ a registered design professional to perform structural observations as defined in Section 1702A.

At the conclusion of the work included in the permit, the structural observer shall submit to the building official a written statement that the site visits have been made and identify any reported deficiencies that, to the best of the structural observer's knowledge, have not been resolved.

1709A.2 Structural observations for seismic resistance. *(Relocated from 1702A.2, CBC 2001) Observation of the construction shall be provided by the architect or engineer in general responsible charge as set forth in Title 24, Part 1 Sections 4-333 and 4-344.*

Structural observations shall be provided for those structures included in Seismic Design Category D, E or F, as determined in Section 1613, where one or more of the following conditions exist:

1. The structure is classified as Occupancy Category III or IV in accordance with Section 1604A.5.
2. The height of the structure is greater than 75 feet (22 860 mm) above the base.
3. The structure is assigned to Seismic Design Category E, is classified as Occupancy Category I or II in accordance with Section 1604.5 and is greater than two stories in height.

4. When so designated by the registered design professional in responsible charge of the design.

5. When such observation is specifically required by the building official.

1709A.3 Structural observations for wind requirements. (Relocated from 1702A.2, CBC 2001) *Observation of the construction shall be provided by the architect or engineer in general responsible charge as set forth in Title 24, Part 1 Sections 4-333 and 4-341.*

Structural observations shall be provided for those structures sited where the basic wind speed exceeds 110 mph (49 m/s), determined from Figure 1609, where one or more of the following conditions exist:

1. The structure is classified as Occupancy Category III or IV in accordance with Table 1604.5.

2. The building height is greater than 75 feet (22 860 mm).

3. When so designated by the registered design professional in responsible charge of the design.

4. When such observation is specifically required by the building official.

SECTION 1710A - DESIGN STRENGTHS OF MATERIALS

1710A.1 Conformance to standards. The design strengths and permissible stresses of any structural material that are identified by a manufacturer's designation as to manufacture and grade by mill tests, or the strength and stress grade is otherwise confirmed to the satisfaction of the building official, shall conform to the specifications and methods of design of accepted engineering practice or the approved rules in the absence of applicable standards.

1710A.2 New materials. For materials that are not specifically provided for in this code, the design strengths and permissible stresses shall be established by tests as provided for in Section 1711A.

SECTION 1711A - ALTERNATIVE TEST PROCEDURE

1711A.1 General. In the absence of approved rules or other approved standards, the building official shall make, or cause to be made, the necessary tests and investigations; or the building official shall accept duly authenticated reports from approved agencies in respect to the quality and manner of use of new materials or assemblies as provided for in Section 104.11, *Appendix Chapter 1*. The cost of all tests and other investigations required under the provisions of this code shall be borne by the permit applicant.

SECTION 1712A - TEST SAFE LOAD

1712A.1 Where required. Where proposed construction is not capable of being designed by approved engineering analysis, or where proposed construction design method does not comply with the applicable material design standard, the system of construction or the structural unit and the connections shall be subjected to the tests prescribed in Section 1714A. The building official shall accept certified reports of such tests conducted by an approved testing agency, provided that such tests meet the requirements of this code and approved procedures.

SECTION 1713A - IN-SITU LOAD TESTS

1713A.1 General. Whenever there is a reasonable doubt as to the stability or load-bearing capacity of a completed building, structure or portion thereof for the expected loads, an engineering assessment shall be required. The engineering assessment shall involve either a structural analysis or an in-situ load test, or both. The structural analysis shall be based on actual material properties and other as-built conditions that affect stability or load-bearing capacity, and shall be conducted in accordance with the applicable design standard. If the structural assessment determines that the load-bearing capacity is less than that required by the code, load tests shall be conducted in accordance with Section 1713A.2. If the building, structure or portion thereof is found to have inadequate stability or load-bearing capacity for the expected loads, modifications to ensure structural adequacy or the removal of the inadequate construction shall be required.

1713A.2 Test standards. Structural components and assemblies shall be tested in accordance with the appropriate material standards listed in Chapter 35. In the absence of a standard that contains an applicable load test procedure, the test procedure shall be developed by a registered design professional and approved. The test procedure shall simulate loads and conditions of application that the completed structure or portion thereof will be subjected to in normal use.

1713A.3 In-situ load tests. In-situ load tests shall be conducted in accordance with Section 1713A.3.1 or 1713A.3.2 and shall be supervised by a registered design professional. The test shall simulate the applicable loading conditions specified in Chapter 16A as necessary to address the concerns regarding structural stability of the building, structure or portion thereof.

1713A.3.1 Load test procedure specified. Where a standard listed in Chapter 35 contains an applicable load test procedure and acceptance criteria, the test procedure and acceptance criteria in the standard shall apply. In the absence of specific load factors or acceptance criteria, the load factors and acceptance criteria in Section 1713A.3.2 shall apply.

1713A.3.2 Load test procedure not specified. In the absence of applicable load test procedures contained within a standard referenced by this code or acceptance criteria for a specific material or method of construction, such existing structure shall be subjected to a test procedure developed by a registered design professional that simulates applicable loading and deformation conditions. For components that are not a part of the seismic-load-resisting system, the test load shall be equal to two times the unfactored design loads. The test load shall be left in place for a period of 24 hours. The structure shall be considered to have successfully met the test requirements where the following criteria are satisfied:

1. Under the design load, the deflection shall not exceed the limitations specified in Section 1604A.3.
2. Within 24 hours after removal of the test load, the structure shall have recovered not less than 75 percent of the maximum deflection.
3. During and immediately after the test, the structure shall not show evidence of failure.

SECTION 1714A - PRECONSTRUCTION LOAD TESTS

1714A.1 General. In evaluating the physical properties of materials and methods of construction that are not capable of being designed by approved engineering analysis or do not comply with applicable material design standards listed in Chapter 35, the structural adequacy shall be predetermined based on the load test criteria established in this section.

1714A.2 Load test procedures specified. Where specific load test procedures, load factors and acceptance criteria are included in the applicable design standards listed in Chapter 35, such test procedures, load factors and acceptance criteria shall apply. In the absence of specific test procedures, load factors or acceptance criteria, the corresponding provisions in Section 1714A.3 shall apply.

1714A.3 Load test procedures not specified. Where load test procedures are not specified in the applicable design standards listed in Chapter 35, the load-bearing and deformation capacity of structural components and assemblies shall be determined on the basis of a test procedure developed by a registered design professional that simulates applicable loading and deformation conditions. For components and assemblies that are not a part of the seismic-load-resisting system, the test shall be as specified in Section 1714A.3.1. Load tests shall simulate the applicable loading conditions specified in Chapter 16A.

1714A.3.1 Test procedure. The test assembly shall be subjected to an increasing superimposed load equal to not less than two times the superimposed design load. The test load shall be left in place for a period of 24 hours. The tested assembly shall be considered to have successfully met the test requirements if the assembly recovers not less than 75 percent of the maximum deflection within 24 hours after the removal of the test load. The test assembly shall then be reloaded and subjected to an increasing superimposed load until either structural failure occurs or the superimposed load is equal to two and one-half times the load at which the deflection limitations specified in Section 1714A.3.2 were reached, or the load is equal to two and one-half times the superimposed design load. In the case of structural components and assemblies for which deflection limitations are not specified in Section

1714A.3.2, the test specimen shall be subjected to an increasing superimposed load until structural failure occurs or the load is equal to two and one-half times the desired superimposed design load. The allowable superimposed design load shall be taken as the lesser of:

1. The load at the deflection limitation given in Section 1714A.3.2.
2. The failure load divided by 2.5.
3. The maximum load applied divided by 2.5.

1714A.3.2 Deflection. The deflection of structural members under the design load shall not exceed the limitations in Section 1604A.3.

1714A.4 Wall and partition assemblies. Load-bearing wall and partition assemblies shall sustain the test load both with and without window framing. The test load shall include all design load components. Wall and partition assemblies shall be tested both with and without door and window framing.

1714A.5 Exterior window and door assemblies. The design pressure rating of exterior windows and doors in buildings shall be determined in accordance with Section 1714A.5.1 or 1714A.5.2.

Exception: Structural wind load design pressures for window units smaller than the size tested in accordance with Section 1714A.5.1 or 1714A.5.2 shall be permitted to be higher than the design value of the tested unit provided such higher pressures are determined by accepted engineering analysis. All components of the small unit shall be the same as the tested unit. Where such calculated design pressures are used, they shall be validated by an additional test of the window unit having the highest allowable design pressure.

1714A.5.1 Exterior windows and doors. Exterior windows and sliding doors shall be tested and labeled as conforming to AAMA/WDMA/CSA101/I.S.2/A440. The label shall state the name of the manufacturer, the approved labeling agency and the product designation as specified in AAMA/WDMA/CSA101/I.S.2/A440. Exterior side-hinged doors shall be tested and labeled as conforming to AAMA/WDMA/CSA101/I.S.2/A440 or comply with Section 1714A.5.2. Products tested and labeled as conforming to AAMA/WDMA/CSA 101/I.S.2/A440 shall not be subject to the requirements of Sections 2403.2 and 2403.3.

1714A.5.2 Exterior windows and door assemblies not provided for in Section 1714A.5.1. Exterior window and door assemblies shall be tested in accordance with ASTM E 330. Exterior window and door assemblies containing glass shall comply with Section 2403. The design pressure for testing shall be calculated in accordance with Chapter 16A. Each assembly shall be tested for 10 seconds at a load equal to 1.5 times the design pressure.

1714A.6 Test specimens. Test specimens and construction shall be representative of the materials, workmanship and details normally used in practice. The properties of the materials used to construct the test assembly shall be determined on the basis of tests on samples taken from the load assembly or on representative samples of the materials used to construct the load test assembly. Required tests shall be conducted or witnessed by an approved agency.

SECTION 1715A - MATERIAL AND TEST STANDARDS

1715A.1 Test standards for joist hangers and connectors.

1715A.1.1 Test standards for joist hangers. The vertical load-bearing capacity, torsional moment capacity and deflection characteristics of joist hangers shall be determined in accordance with ASTM D 1761 using lumber having a specific gravity of 0.49 or greater, but not greater than 0.55, as determined in accordance with AF&PA NDS for the joist and headers.

Exception: The joist length shall not be required to exceed 24 inches (610 mm).

1715A.1.2 Vertical load capacity for joist hangers. The vertical load capacity for the joist hanger shall be determined by testing a minimum of three joist hanger assemblies as specified in ASTM D 1761. If the ultimate vertical load for any one of the tests varies more than 20 percent from the average ultimate vertical load, at least

three additional tests shall be conducted. The allowable vertical load of the joist hanger shall be the lowest value determined from the following:

1. The lowest ultimate vertical load for a single hanger from any test divided by three (where three tests are conducted and each ultimate vertical load does not vary more than 20 percent from the average ultimate vertical load).
2. The average ultimate vertical load for a single hanger from all tests divided by three (where six or more tests are conducted).
3. The average from all tests of the vertical loads that produce a vertical movement of the joist with respect to the header of 0.125 inch (3.2 mm).
4. The sum of the allowable design loads for nails or other fasteners utilized to secure the joist hanger to the wood members and allowable bearing loads that contribute to the capacity of the hanger.
5. The allowable design load for the wood members forming the connection.

1715A.1.3 Torsional moment capacity for joist hangers. The torsional moment capacity for the joist hanger shall be determined by testing at least three joist hanger assemblies as specified in ASTM D 1761. The allowable torsional moment of the joist hanger shall be the average torsional moment at which the lateral movement of the top or bottom of the joist with respect to the original position of the joist is 0.125 inch (3.2 mm).

1715A.1.4 Design value modifications for joist hangers. Allowable design values for joist hangers that are determined by Item 4 or 5 in Section 1715A.1.2 shall be permitted to be modified by the appropriate duration of loading factors as specified in AF&PA NDS but shall not exceed the direct loads as determined by Item 1, 2 or 3 in Section 1715A.1.2. Allowable design values determined by Item 1, 2 or 3 in Section 1715A.1.2 shall not be modified by duration of loading factors.

1715A.2 Concrete and clay roof tiles.

1715A.2.1 Overturning resistance. Concrete and clay roof tiles shall be tested to determine their resistance to overturning due to wind in accordance with SBCCI SSTD 11 and Chapter 15.

1715A.2.2 Wind tunnel testing. When roof tiles do not satisfy the limitations in Chapter 16A for rigid tile, a wind tunnel test shall be used to determine the wind characteristics of the concrete or clay tile roof covering in accordance with SBCCI SSTD 11 and Chapter 15.

Notation [For DSA-SS]:

Authority: Education Code Sections 17310, 81142; Health & Safety Code Section 16022

Reference(s): Education Code Sections 17280 - 17317, and 811130 - 81149; Health & Safety Code Sections 16000 - 16023

Notation [For OSHPD]:

Authority: Health and Safety Code Section 129850

Reference: Health and Safety Code Sections 1275, 129850 and 129790